

ORDRE DES INGÉNIEURS DU QUÉBEC

NOVEMBER 2011 SESSION

Open book examination

Calculators : only authorized models

Duration : 3 hours

98-CIV-B4 ENGINEERING HYDROLOGY

**Question 1 (20 pts)**

A time series of maximum annual flows  $Q$  of the Noire River covers years 1961-2000. The following statistics were retrieved from this 40-year time series:

$$Q_{\text{mean}} = 550 \text{ m}^3/\text{s}$$

$$s = 155 \text{ m}^3/\text{s}$$

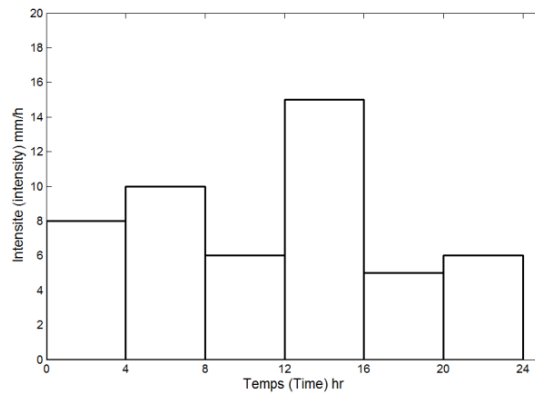
where  $Q_{\text{mean}}$  is the average of the maximal annual flows and  $s$  is the standard deviation.

A histogram plot of the time series confirms that  $Q$  follows a normal distribution.

- Compute the maximal annual flow corresponding to a return period of 100 years. (5 pts)
- What is the probability that the maximal annual flow will be between 700 and 800  $\text{m}^3/\text{s}$ ? (5 pts)
- The municipality of Belle Rivière gets flooded when the river flow exceeds 850  $\text{m}^3/\text{s}$ . Compute the return period associated with this event. (5 pts)
- A time series of flows of the Noire River measured between 1921 and 1960 was found in archives.  $Q_{\text{mean}}$  and  $s$  of this time series is 520 and 145  $\text{m}^3/\text{s}$ , respectively. Verify the hypothesis that  $Q_{\text{mean}}$  computed of the 1961-2000 period is statistically similar to  $Q_{\text{mean}}$  of the 1921-1960 period. (5 pts)

### Question 2 (20 pts)

A raingage recorded the hyetograph below, corresponding to a 24-hour rainfall event:



This rainfall event occurred uniformly over a watershed having a surface area of 295 ha.

The total runoff volume measured at the watershed outlet is  $75000 \text{ m}^3$ . Base flow in the river draining the watershed was constant at  $0.3 \text{ m}^3/\text{s}$  during the storm. Compute the SCS runoff curve number (CN) of this watershed.

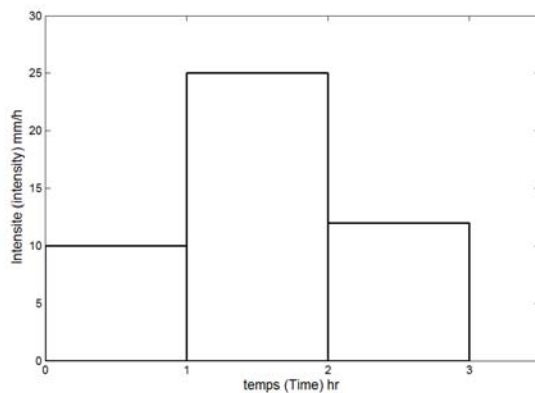
### Question 3 (20 pts)

Calculate the net rainfall hyetograph from the total rainfall hyetograph below, using the following infiltration parameters of the Horton model:

$$f_o = 30 \text{ mm/h}$$

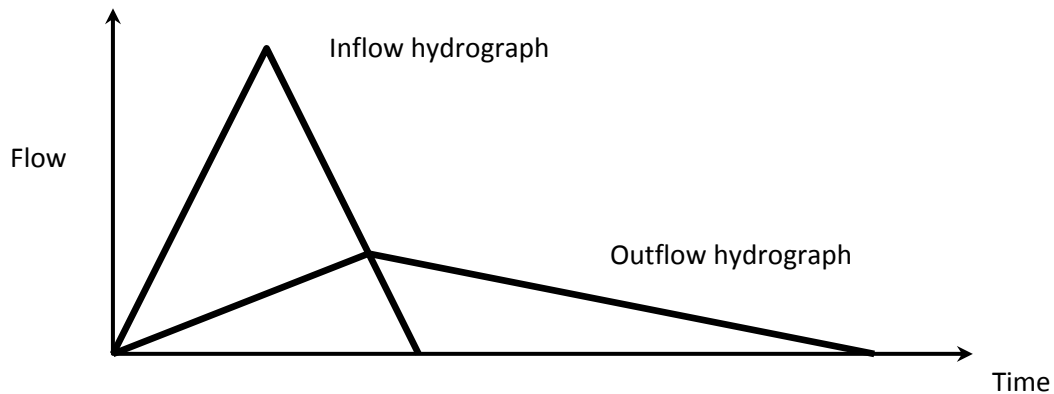
$$f_c = 5 \text{ mm/h}$$

$$k = 1.5 \text{ h}^{-1}$$



#### Question 4 (20 pts)

The volume of an existing detention basin with vertical walls is  $10000 \text{ m}^3$ . It is proposed to replace the current outlet by a smaller one because of recurring flooding problems downstream from the basin. Maximum flow at the basin outlet will be  $0.5 \text{ m}^3/\text{s}$ . The inflow and outflow hydrographs are approximated by triangular shapes as presented in the figure below.



For safety reasons, the time to fill/empty the detention basin has been established to 10 hours.

- What is the maximum peak flow of the inflow hydrograph that the detention basin can accept? Base flow of the inflow hydrograph is 1.5 hour. (10 pts)
- The watershed that generates the runoff entering the detention basin has an area of 44 ha, a time of concentration of 12 minutes and a runoff coefficient  $C=0.35$ . Calculate the return period of the maximum peak flow entering the detention basin. Use the idf curves below for the computations. (10 pts)

$$i = \frac{a}{t + c} \quad t \text{ in minutes, } i \text{ in mm/h}$$

T (years)	c	a (mm/h)
2	13	1778
5	13	2464
10	16	2819
25	24	4318
50	27	4750
100	28	5588

#### Question 5 (20 pts)

Compute and plot the direct runoff hydrograph resulting from the convolution of the unit hydrograph and the total runoff hyetograph below. What are the peak flow  $Q_p$ , time to peak  $t_p$  and base flow  $t_B$  of the direct runoff hydrograph?

The unit hydrograph is for a net rainfall of 1 cm depth and 3 hour duration.

It is assumed that the initial soil infiltration capacity is 14 mm/h and decreases linearly to reach 2 mm/h after 12 hours.

**Unit hydrograph**

Time (hours)	HU ( $m^3/s$ )
0	0
3	40
6	120
9	90
12	50
15	0

**Total rainfall hyetograph**

Time (hours)	Precipitation (mm/h)
0h00- 3h00	15.5
3h00- 6h00	4
6h00- 9h00	3
9h00- 12h00	7.5